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ANALYTICAL AND EXPERIMENTAL INVESTIGATION OF THE EFFECT OF TUNED VISCOELASTIC DAMPERS ON RESPONSE OF SIMPLE BEAMS WITH VARIOUS BOUNDARY CONDITIONS

D. I. G. JONES

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FOREWORD

This report was prepared by the Strength and Dynamics Branch, Metals and Ceramics Division, under Project No. 7351, "Metallic Materials," Task No. 735106, "Behavior of Metals". This research work was conducted in the Air Force Materials Laboratory, Research and Technology Division, Wright-Patterson Air Force Base, Ohio by Dr. D. I. C. Jones.

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This technical report has been reviewed and is approved.

W. J. TRAPP

Chief, Strength and Dynamics Branch Metals and Ceramics Division Air Force Materials Laboratory

ABSTRACT

An approximate analysis of the response in the fundamental mode of any simple single span beam with tuned viscoelastic dampers attached at discrete locations to a harmonic loading with arbitrary spatial distribution is derived. is shown that, to a good degree of approximation, a single expression can be made to represent the response in the fundamental mode of a beam with any boundary conditions, provided that certain effective mass and stiffness parameters are defined for the beam-damper configuration. Comparisons are made with experiments and with an exact theory, subject to the limitations of the Euler-Bernoulli beam equation, of the response and damping of a cantilever beam having an isolated harmonically varying load at the free end and a clampedclamped beam, with a tuned damper at the center, under shaker excitation. Good agreement between the exact and approximate theories and the experiments is demonstrated. Conclusions are drawn concerning the equivalent damping introduced into the simple structure by the tuned dampers and the damper natural frequency needed for optimal damping.

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LIST OF SYMBOLS

Λ	Resonant amplification factor (shaker excitation)
exp	Exponential function
E	Young's Modulus for beam material
Fj	Force transmitted back to structure by j th damper unit
i	Square root of minus 1
I	Second moment of area of beam cross section
σ	Total number of dampers on beam
k	Real part of stiffness of damper unit
L	Length of beam
m	Tuning mass of damper unit. Also dummy subscript
P(x)	Amplitude of transverse applied loading
P _n	n th term in expansion of $P(x)$ as series of normal modes
D	Resonant amplification factor (force excitation)
t	Time
$W(\mathbf{x})$	Amplitude of transverse displacement of beam
W n	n th term in expansion of $V(\mathbf{x})$ as series of normal modes
X ·	Station along beam
×j	Station of j th damper
α	See equation (22)
β	See equation (23)
Υ	See equation (24)
Γ	kL ³ /EIξ ₁ ⁴ - non-dimensional stiffness parameter
re	See equation (12)
δ	Dirac Delta function
Δ	x/L

Δj ×i/L Loss factor of viscoelastic spring of damper unit η Effective loss factor of beam-damper configuration $[=(Q^2-1)^{-1/2}]$ ηs Mass per unit length of beam μ $(\mu\omega^2L^4/EI)^{1/4}$ - frequency parameter ξ $(\mu\omega^2L^4/EI)^{1/4}$ - n th eigenvalue of undamped beam ξ_n n th normal mode of undamped beam m/µL - non-dimensional mass parameter See equation (11) Circular frequency Natural circular frequency of n th normal mode of ω_n undamped beam = $(k/m)^{1/2}$ - natural frequency of damper unit WD

Suffixes:

subscript referring to damper

subscript referring to reduction of data for all boundary conditions

j subscript denoting j th damper

n subscript referring to number of normal mode

subscript referring to effective damping for beam-damper system

I. INTRODUCTION

The tuned damper, consisting of a spring-dashpot combination (or a viscoelastic spring) connecting a mass to a point on a vibrating structure, has recently been examined from the point of view of a possible application to the damping of complex structures exhibiting closely spaced resonant frequencies [1, 2]. Since analysis is usually difficult in such cases investigations of the effect of tuned dampers on the response of simpler structures have served as essential preliminaries. Such analyses have been carried out by Snowdon [3] and others [4-7].

However, although exact solutions within the framework of the Euler-Bernoulli equation have been obtained for several beam-damper configurations, under various harmonic loadings, no attempt has apparently been made to derive a general theory, applicable equally to all simple beam structures and harmonic loadings. Such a theory is developed in this paper. An approximate theory, applicable for the fundamental mode primarily, is obtained for a simple beam with tuned dampers at various points and subjected (i) to a harmonic loading of arbitrary spatial dependence and (ii) to displacement (shaker) excitation at the support(s). It is shown that a single expression can be made to represent the transmissibility for all boundary conditions. Comparisons are made with exact solutions obtained for a cantilever beam, harmonically loaded by a force at the free end, and with a tuned damper at the free end [4] and for a clampedclamped beam with a tuned damper at the center, subjected to displacement (shaker) excitation at the supports [5]. The exact and approximate theories are shown to be in good agreement.

Experimental investigations of cantilever and clampedclamped beams, with an isolated tuned damper at the free end and at the center respectively, are described. It is shown that the main conclusions of the various theories are borne out.

II. APPROXIMATE ANALYSIS OF TUNED DAMPERS ON SINGLE SPAN BEAM UNDER FORCE EXCITATION

Consider a single span beam of length L with tuned visco-elastic dampers of complex spring stiffness $k(1+i\eta)$ and mass m at a number of points x=x, $(j=1\ to\ J)$ as in Figure 1. The amplitude of the harmonic j force transmitted back to the structure (F_j) by the damper at the point x_j is readily obtained $[4,\ 5]$ by j solving the equation of motion of the mass m for the damper subjected to a harmonic input displacement of amplitude $W(x_j)$ at the point of attachment to the beam. Then:

$$F_{j} = \frac{- m\omega^{2} W(x_{j}) \delta(x - x_{j})}{1 - m\omega^{2} / k(1 + i\eta)}$$
(1)

The Euler-Bernoulli equation for the beam under the action of a harmonic loading of amplitude P(x) is therefore written:

$$EI(d^{4}W/dx^{4}) - \mu\omega^{2}W - \frac{m\omega^{2}}{1-m\omega^{2}/k(1+i\eta)} \int_{j=1}^{J} W(x_{j}) \delta(x-x_{j}) = P(x)$$
(2)

If W(x) and P(x) are now expanded as series of normal modes of the undamped beam, assumed to be known, then these modes must satisfy the homogeneous equation of motion;

$$d^{4} \Phi_{n}(x) / dx^{4} - (\mu \omega_{n}^{2} / EI) \Phi_{n}(x) = 0$$
 (3)

If use is made of this fact, equation (2) may be written:

$$\sum_{n=1}^{\infty} (\mu \omega_n^2 - \mu \omega^2) W_n \Phi_n(x/L) - \frac{m \omega^2}{1 - m \omega^2 / k(1 + i \eta)} \sum_{j=1}^{J} \delta(x - x_j) \sum_{m=1}^{\infty} W_m \Phi_m(x_j/L)$$

$$= \sum_{n=1}^{\infty} P_n \Phi_n(x/L)$$
 (4)

where
$$W(x) = \sum_{n=1}^{\infty} W_n \Phi_n(x/L)$$
 (5)

and
$$P(x) = \sum_{n=1}^{\infty} P_n \Phi_n(x/L)$$
 (6)

If we now factor all the terms of equation (4) by $\Phi_n(x/L)$ and integrate with respect to x from 0 to L, then:

$$(\mu \omega_n^2 - \mu \omega^2) W_n = \int_0^L \Phi_n^2(x/L) dx - P_n = \int_0^L \Phi_n^2(x/L) dx$$

$$-\frac{m\omega^2}{1-m\omega^2/k(1+i\eta)}\sum_{j=1}^{J}\Phi_n(x_j/L)\sum_{m=1}^{\infty}W_m\Phi_m(x_j/L)=0$$
 (7)

use being made of the orthogonal property of the normal modes i.e.

$$\int_{0}^{L} \Phi_{m}(x/L) \Phi_{n}(x/L) dx = 0 \quad (m \neq n)$$
(8)

In this particular investigation, one is interested primarily in the response in the vicinity of the fundamental mode of the beam, for which n=1. Therefore:

$$W_{1} = \frac{P_{1}L^{4}/EI}{\int_{j=1}^{J} \Phi_{1}(\Delta_{j}) \sum_{m=1}^{\infty} (W_{m}/W_{1}) \Phi_{m}(\Delta_{j})} (9)$$

$$[1-\psi\xi^{4}/\Gamma(1+i\eta)] \int_{0}^{1} \Phi_{1}^{2}(\Delta) d\Delta$$

It is clear from equation (7) that, since for any simple beam $\omega_n^2 >> \omega_1^2 \ (n>1) \,, \ \, W_n << W_1 \quad \mbox{in the vicinity of the first resonant} \label{eq:weak_problem}$ frequencies of the beam-damper system. Therefore only the first term in the series with respect to m in equation (9) need be retained so that, at the point of maximum amplitude where $\phi_1 = 1$,

$$\frac{\text{EI}\xi_{1}^{4} \text{ W}}{\text{P}_{1}^{\text{L}^{4}}} = \frac{1}{1 - (\xi/\xi_{1})^{4} - \frac{\psi_{e}(\xi/\xi_{1})^{4}}{1 - \psi_{e}(\xi/\xi_{1})^{4}/\Gamma_{e}(1 + i\eta)}}$$
(10)

where $\psi_{\mathbf{p}}$ is an effective mass parameter defined by

$$\psi_{e} = \psi_{j=1}^{J} \Phi_{1}^{2}(\Delta_{j}) / \int_{0}^{1} \Phi_{1}^{2}(\Delta) d\Delta$$
 (11)

and

$$\Gamma_{e} = \Gamma \sum_{j=1}^{J} \Phi_{1}^{2}(\Delta_{j}) / \int_{0}^{1} \Phi_{1}^{2}(\Delta) d\Delta$$
 (12)

is an effective stiffness parameter. It has therefore been shown that the theory of the response of any simple beam, for which the resonant frequencies are sufficiently well separated for certain approximations to be made, can be reduced to a single expression if appropriate effective mass and stiffness parameters are defined for each particular set of boundary conditions. Certainly, the integrals and summations in equations (11) and (12) are readily evaluated for most cases using the tables of normal modes given by Bishop and Johnson [8]. Some of the integrals and summations are given in Table A for a number of boundary conditions.

It will be seen that equation (10) is the same as that obtained if one had assumed that the beam was uniformly covered by a distribution of tuned dampers with the effective mass parameter ψ_e now representing the true mass ratio for the distributed dampers i.e. the ratio of the total mass of the dampers to the total mass of the beam [9]. This is apparent from equation (11) when j approaches infinity. Similarly, Γ_e is seen to be equal to $L^3/\text{EI}\xi_1^4$ times the total stiffness of all the damper springs in parallel. The theory of the beam with distributed tuned dampers has already been developed [9, 10].

On the basis of equation (10), the amplitude |W| of the response can readily be determined for various specific values of $\psi_{\mathbf{e}}$, $\Gamma_{\mathbf{e}}$ and η as a function of ξ/ξ_1 or $(\xi/\xi_1)^2$. Typical graphs of $(\text{EI}\,\xi_1^4/P_1\text{L}^4)$ |W| are plotted against $(\xi/\xi_1)^2$, which is proportional to the frequency ω , in Figures 2 and 3. Further data and graphs are available [9] for other values of η .

From the response spectra which have essentially two resonant peaks, a measure of the performance of the dampers in damping the beam is given by an arbitrarily defined effective loss factor $\mathbf{n}_{_{\mathbf{S}}}$, defined by $\mathbf{n}_{_{\mathbf{S}}}=(Q^2-1)^{-1/2}$, where Q is the amplification factor of each resonant peak. Computed values of Q at resonance are given in Table II. Typical graphs of $\mathbf{n}_{_{\mathbf{S}}}$ against the effective stiffness parameter $\mathbf{r}_{_{\mathbf{C}}}$ are plotted in Figure 4 for the high and low frequency resonant peaks. Further data for other values of $\psi_{_{\mathbf{C}}}$ are available [9].

At the point where the two resonance peaks are of equal amplitude, the dampers are said to be optimally tuned [3, 6]. This is the point at which the curves of η against Γ_e cross over in Figure 4. At all other values of Γ_e , one or other of the two resonance peaks will have a higher amplification factor Q than at the point of optimal tuning. Typical graphs of the value of η_s for optimally tuned dampers are plotted in Figure 5 against the parameter ψ_e . The data is taken from Table III, where values of η_s and Γ_e are given for various ψ_e and η for both the exact theory (discussed later) and the present approximate theory. These tabulated values are taken from graphs such as Figure 4. A cross plot of the data given in Figure 5 gives η_s as a function of η for various ψ_e , and this data is plotted in Figure 6.

The values of $\Gamma_{\rm e}$ at which the dampers are optimally tuned are also of great interest since, from the definition of $\Gamma_{\rm e}$:

$$\Gamma_{e} = (kL^{3}/EI\xi_{1}^{4}) \sum_{j=1}^{J} \Phi_{1}^{2}(\Delta_{j}) / \int_{0}^{1} \Phi_{1}^{2}(\Delta) d\Delta$$

$$= (\omega_{D}/\omega_{1})^{2} \psi_{e}$$

$$\omega_{D}/\omega_{1} = (\Gamma_{e}/\psi_{e})^{1/2}$$
(13)

It is, therefore, a simple matter to determine the ratio of the natural frequency ω_D of the damper to the natural frequency ω_1 of the undamped beam from the values of Γ_e at the point of optimal tuning. A typical graph of ω_D/ω_1 against ψ_e is shown in Figure 7. Of more interest, however, is the graph of $(\omega_D/\omega_1)\,(1+\psi_e)^{1/2}\,(1+\eta^2)^{1/4}$, for the exact and approximate theories, against ψ_e plotted for several values of the damper loss factor η in Figure 8. This empirically derived representation collapses all the data on to a single straight line so that the relationship between ω_D/ω_1 and ψ_e and η is:

$$\omega_{\rm D}/\omega_1 = (1+\psi_{\rm e})^{-1/2} (1+\eta^2)^{-1/4}$$
 (14)

Equation (14) implies that, if ψ_e and η are known, it is possible to determine the natural frequency ω_D of the damper such that the beam-damper system is optimally damped. This simple relationship should therefore be of value for simple systems exhibiting widely separated resonance frequencies and may serve as a guide for more complex structures to which it is desired to attach tuned viscoelastic dampers (See [2]).

III. APPROXIMATE ANALYSIS OF TUNED DAMPERS ON SINGLE SPAN BEAM UNDER SHAKER EXCITATION

If U is the amplitude of transverse displacement of any point x of the beam relative to the clamped end or ends (the analysis must clearly be limited to cases where at least one end of the beam is attached to the shaker), the equation of motion may be written:

$$EI(d^4U/dx^4) - \mu\omega^2[U+X]$$

$$-\frac{m\omega^{2}}{1-m\omega^{2} / k(1+i\eta)} \int_{j=1}^{J} [U(x_{j})+X] \delta(x-x_{j}) = 0$$
(15)

which may also be written:

$$\text{EI}\left(d^4U/dx^4\right) - \mu\omega^2U - \frac{m\omega^2}{1-m\omega^2 \text{ / k(1+in)}} \quad \sum_{j=1}^{J} \quad U(x_j) \, \delta(x-x_j)$$

$$= \mu \omega^{2} X + \frac{m \omega^{2} X}{1 - m \omega^{2} / k (1 + i \eta)} \int_{j=1}^{J} \delta(x - x_{j})$$
 (16)

This equation is clearly different from equation (2) but may be solved in much the same way. Again, we replace the response U(x) by the appropriate expansion in normal modes. Then:

$$U(\mathbf{x}) = \sum_{n=1}^{\infty} U_n \Phi_n(\mathbf{x}/\mathbf{L})$$
 (17)

and, using equation (3) which applies equally to this case, equation (16) becomes:

$$(\xi_{n}^{4}-\xi^{4})\sum_{n=1}^{\infty}U_{n}^{\Phi}_{n}(x/L)-\frac{m\omega^{2}L^{4}/EI}{1-m\omega^{2}/k(1+i\eta)}\sum_{m=1}^{\infty}U_{m}^{\Phi}_{m}(x_{j}/L)\sum_{j=1}^{J}\delta(x-x_{j})$$

$$= \xi^{4}X + \frac{m\omega^{2}L^{4}/EI}{1-m\omega^{2}/k(1+i\eta)} \sum_{j=1}^{J} \delta(x-x_{j})$$
 (18)

If we factor both sides of equation (18) by $\Phi_n(x/L)$, integrate from 0 to L with respect to x and make use of orthogonal property of the normal modes:

$$(\xi_{n}^{4}-\xi^{4})U_{n}\int_{0}^{1}\Phi_{n}(\Delta)d\Delta - \frac{\psi\xi^{4}}{1-\psi\xi^{4}/\Gamma(1+i\eta)}\sum_{m=1}^{\infty}U_{m}\Phi_{m}(\Delta_{j})\sum_{j=1}^{J}\Phi_{n}(\Delta_{j})$$

$$= \xi^{4}X \int_{0}^{1} \Phi_{n}(\Delta) d\Delta + \frac{\psi \xi^{4}}{1 - \psi \xi^{4} / \Gamma(1 + i\eta)} \int_{j=1}^{J} \Phi_{n}(\Delta_{j})$$
 (19)

Considering the first mode only, therefore:

$$U_{1} \left[\xi_{1} - \xi^{4} - \frac{\psi \xi^{4} \sum_{j=1}^{J} \Phi_{1} (\Delta_{j}) \sum_{m=1}^{\infty} (U_{m}/U_{1}) \Phi_{m}(\Delta_{j})}{[1 - \psi \xi^{4} / \Gamma (1 + i \eta)] \int_{0}^{1} \Phi_{1}^{2} (\Delta) d\Delta} \right]$$

$$= X \frac{\xi^{4} \int_{0}^{1} \Phi_{1}(\Delta) d\Delta}{\int_{0}^{1} \Phi_{1}^{2} (\Delta) d\Delta} + \frac{\psi \xi^{4} \sum_{j=1}^{J} \Phi_{1}(\Delta_{j})}{\int_{0}^{1} \Phi_{1}^{2} (\Delta) d\Delta}$$

$$= \frac{1}{\int_{0}^{1} \Phi_{1}^{2} (\Delta) d\Delta} + \frac{\psi \xi^{4} \int_{0}^{1} \Phi_{1}(\Delta_{j})}{\int_{0}^{1} \Phi_{1}^{2} (\Delta) d\Delta}$$

$$(20)$$

and, since $U_n \ll U_1$ in the neighborhood of the fundamental frequency, we may write as an approximation:

$$\frac{U_{1}}{x} = \frac{(\xi/\xi_{1})^{4}\alpha + \frac{\psi(\xi/\xi_{1})^{4}\beta}{1-\psi(\xi/\xi_{1})^{4}/\Gamma(1+i\eta)}}{1-\psi_{e}(\xi/\xi_{1})^{4}/\Gamma(1+i\eta)}$$

$$\frac{\mathbf{r}}{\mathbf{x}} = \frac{\mathbf{U}_{1} + \mathbf{x}}{\mathbf{x}} + (\xi/\xi_{1}^{4}) \left[\frac{\alpha + \beta \psi (\xi/\xi_{1})^{4} / \{1 - \psi (\xi/\xi_{1})^{4} / \Gamma(1 + i\eta)\}}{1 - (\xi/\xi_{1})^{4} - \gamma \psi (\xi/\xi_{1})^{4} / [1 - \psi (\xi/\xi_{1})^{4} / \Gamma(1 + i\eta)]} \right]$$

(21)

where
$$\alpha = \int_0^1 \Phi_1(\Delta) \Phi \Delta / \int_0^1 \Phi_1^2(\Delta) d\Delta$$
 (22)

and
$$\beta = \sum_{j=1}^{J} \Phi_{1}(\Delta_{j}) / \int_{0}^{1} \Phi_{1}^{2} (\Delta_{j}) d\Delta$$
 (23)

and
$$\gamma = \sum_{j=1}^{J} \Phi_{1}^{2}(\Delta_{j}) / \int_{0}^{1} \Phi_{1}^{2}(\Delta) d\Delta$$
 (24)

It is seen that the response is now governed by two additional parameters, namely α and β . In the special case where $J^{+\infty}$, i.e. the dampers are uniformly distributed, $\alpha+\beta$. Also, for J=1 and $\Phi_1(\Delta_j)=1$, i.e. the case where the single damper location and the point at which the mode shape is normalized are identical, then $\beta=\gamma$ also. This particular case is of some importance and analysis will be limited to this case. If $\beta=\gamma$, therefore:

$$\frac{W}{X} = 1 + \left[\frac{\xi}{\xi_1}\right]^4 \left[\frac{\alpha + \psi_e(\xi/\xi_1)^4 / [1 - \psi_e(\xi/\xi_1)^4 / \Gamma_e(1 + i\eta)]}{1 - (\xi/\xi_1)^4 - \psi_e(\xi/\xi_1)^4 / [1 - \psi_e(\xi/\xi_1)^4 / \Gamma_e(1 + i\eta)]}\right]$$

$$= \frac{1 + (\xi/\xi_1)^{4} (\alpha-1)}{1 - (\xi/\xi_1)^{4} - \frac{\psi_e(\xi/\xi_1)^{4}}{1 - \psi_e(\xi/\xi_1)^{4} / \Gamma_e(1+i\eta)}}$$
(25)

In this particularly simple case, therefore, the problem of determining |W/X| under shaker excitation reduces to that of factoring the response under force excitation, given in equation (10) by $1+(\xi/\xi_1)^4$ ($\alpha-1$). For the response determined in this way, two peaks are again observed and it has been shown [4, 5] that, for shaker excitation, the effective loss factor n_g is defined by the relationship:

$$\eta_{S} = \frac{\alpha \phi_{1}(\Delta)}{\sqrt{A^{2}-1}}$$
 (26)

where A is the amplification factor i.e. the value of |W/X| at each resonance in the fundamental modes. Values of the amplification factor A under shaker excitation are given in Table II for a clamped-clamped beam along with values of Q for Force excitation. Typical graphs of η_s defined as in equation 25, against Γ_e are shown in Figure 9. From these graphs, the optimum loss factor corresponding to the point of cross-over, can be read off and plotted against ψ_e for values of η_s . The points are plotted in Figure 5 and show that the variation of η_s with ψ_e is practically independent of whether the beam is force or shaker excited.

On the other hand, graphs of $(\Gamma_e/\psi_e)^{1/2}(1+\psi_e)^{+1/2}(1+\eta^2)^{+1/4}$ and $\omega_D/\omega_1=(\Gamma_e/\psi_e)^{1/2}$ against ψ_e do show some differences, as figures 10 and 11 show.

IV. COMPARISON OF EXACT AND APPROXIMATE ANALYSES

(i) Cantilever beam under force excitation

Previous investigations of tuned dampers on simple beams have led to exact solutions of the Euler-Bernoulli Equation for a beam with a tuned damper at an antinodal point. For example, the response of a cantilever beam with a tuned damper at the free end is described by Young [11] and Nashif [4]. Graphs of (EI/PL 4) |W| against ξ for a load of amplitude P at the free end were obtained from the exact theory [4] and were shown to consist of two resonance peaks in the vicinity of the fundamental mode, as in the approximate theory. Graphs of $\eta_{\rm S}$ against Γ were drawn as for the approximate theory and some of the results are tabulated in Table III for the optimally tuned case where the two resonance peaks are of equal amplitude. Since only one damper was considered for the exact theory [5]

of the cantilever beam, $\sum_{j=1}^{J} \Phi_{1}^{2}(\Delta_{j}) = 1 \text{ and, as in Table I,}$ $\int_{0}^{1} \Phi_{1}^{2}(\Delta) d\Delta = 0.25. \text{ Therefore, for this case, } \psi_{e} = 4\psi \text{ and}$

 $\Gamma_{\rm e}$ = 4 $\Gamma_{\rm e}$. From the values of Γ and ψ [4] therefore, $\Gamma_{\rm e}$ and $\psi_{\rm e}$ were derived and the values of $\eta_{\rm s}$ plotted against $\psi_{\rm e}$ in Figure 5. It is seen that the computed points lie essentially along the same line as given by the approximate theory.

Furthermore, the values of $(\omega_D/\omega_1)(1+\psi_e)^{1/2}(1+\eta^2)^{1/4}$, when plotted against ψ_e , lie on the same straight line as given by the approximate theory, as in Figure 8.

(ii) Clamped-clamped beam under shaker excitation

A previous investigation [5] has given the exact theory of a tuned damper at the center of a clamped-clamped beam on the response under shaker excitation. Some of the results are tabulated in Tables II and IV. Again $\sum_{j=1}^{J} \Phi_{1}^{2}(\Delta_{j}) = 1 \text{ and, }$ as in Table I. $\int_{0}^{1} \Phi_{1}^{2}(\Delta) d\Delta = 0.439.$ Thus, for this case, $\psi_{e} = 2.086\psi \text{ and } \Gamma_{e} = 2.086\Gamma.$ From the values of Γ and ψ [5], Γ_{e} and ψ_{e} were deduced and entered into Table V and graphs of Γ_{e} plotted against ψ_{e} , as in Figure 5. It is seen that the computed points lie along the same curve as all the others.

Values of ω_D/ω_1 and $(\Gamma/\psi\xi_1^4)^{1/2}(1+\psi_e)^{1/2}(1+\eta^2)^{1/4}$ when plotted against ψ_e , lie along the same line as given by the approximate theory in Figures 10 and 11 respectively.

(i) Cantilever beam with distributed tuned dampers under shaker excitation

This investigation has previously been reported in reference [9]. In brief, a cantilever beam with eleven tuned dampers of the geometry shown in Figure 12 was vibrated by an electrodynamic shaker. An accelerometer at the tip was used to measure the response and the effective damping deduced from the appropriate relationship, namely $\eta_s = 1/\sqrt{A^2-1}$.

The length of the beam was varied so as to obtain proper tuning, i.e. to make the two response peaks, corresponding to the fundamental mode of equal amplitude. The loss factor of each damper was determined as in [9] and plotted in Figure 13. Comparison of the measured values of η_s for $\eta \simeq 0.175$ and 0.09 and various values of ψ_e are shown in Figure 14.

It is seen that the agreement between theory and experiment is good.

Another part of the investigation, not previously reported, involved the verification of the relationship given in equation (14) for the point of optimum tuning. The geometry of the dampers used in this investigation is shown in Figure 15 A. The density of the aluminum was 0.101 Lb/in³ and that of the viscoelastic material (LD-400) was 0.0522 Lb/in³. The total weight of the resilient part of the damper up to the last half inch in which the tip mass is situated is:

$$m_D$$
 = 0.5 x 1 x 0.02 x 0.101 x 454
+ 0.0522 x 0.5 x 0.75 x τ x 454
= 0.459 + 8.83 τ gms

where τ is the thickness of the viscoelastic material in inches and the damper breadth is 0.5 inches. Now an additional mass equal to the weight of the outermost half inch of the damper beam must be included. This amounts to 0.5 x 0.5 x 0.02 x 0.101 x 454 x 0.23 gms. The total effective mass to be added to the nominal mass m_{τ} at the free end in order to give the true mass can be shown from [4] to be

$$m = m_t + 0.23 + 0.236 m_D \text{ gms}$$

= $m_t + 0.338 + 2.08\tau \text{ gms}$

 $\omega_D^{m^{1/2}}$ should depend only on τ/h_D for this particular geometry of damper. Tests were carried out for these dampers, with

various masses m_t at the free end under shaker excitation. An accelerometer was placed at the free end and formed part of the mass m_t. Response spectra showing the variation of the acceleration at the free end with frequency for several input accelerations at the clamped end were measured and the frequency at which the output acceleration was greatest noted. Some typical results are shown in Table V. Further tests, in which the response at the tip was measured optically were also carried out, and the results are given in Table VI. From these results, a graph of $\omega_D^{m1/2}$ against τ/h_D was plotted as in Figure 16. It is seen that the data do indeed collapse on to a single curve.

Since optimal tuning was obtained by varying the length of the beam until the two response peaks corresponding to the fundamental mode were of equal amplitude, the fundamental natural frequency of the cantilever beam with no dampers (but with the bolts used to hold the clamps in place) was measured on the shaker and plotted against the beam length L, as in Figure 17.

Table 1 of reference [9] gives the experimental data obtained for the clamped-free beam with eleven distributed dampers under shaker excitation. From the values of L for optimal tuning, ω_1 can be read off Figure 16 and from the values of m_1 (referred to as "m" in Table 1 of reference [9]) we can deduce ω_D . Hence $(\omega_D/\omega_1)^{-1/4}$ (1+ η^2) 1/4 can be calculated for the point of optimum tuning. This calculation is carried out in Table VII. The values of $(\omega_D/\omega_1)^{-1/4}$ e $(1+\eta^2)^{-1/4}$ are plotted against ψ_D in Figure 18.

(ii) Cantilever beam with tuned damper at free end under shaker excitation

This investigation has previously been reported in reference [4]. In this experimental investigation, the effective damping of the setup shown in Figure 19 was determined from the experimental amplification factor A under shaker excitation by means of the relationship given in Equation 26. The damper configuration used is shown in Figure 15B. The length of the beam was varied so as to obtain proper tuning in the fundamental mode. For the point of optimum damping, graphs of the optimum η against ψ were plotted, as in reference [4]. These graphs are re-plotted as graphs of $\eta_{\rm S}$ against ψ for η = 0.22 and η = 0.8 in Figures 20 and 21 respectively. It is seen that the agreement between theory and experiment is satisfactory. The loss factor η of each damper was determined as in [4] and is plotted against $\tau/h_{\rm D}$ in Figure 22.

Measurements of the natural frequencies of the dampers for various tip masses \mathbf{m}_{t} were again made. The value of \mathbf{m}_{D} is, now

$$m_D = 0.5 \times 1 \times 0.02 \times 0.101 \times 454$$

+ $0.0522 \times 0.5 \times 1 \times \tau \times 454$
+ $0.459 + 11.8\tau \text{ gms}$

where τ is again the viscoelastic material thickness in inches.

$$m = m_{t} + 0.23 + 0.236 m_{D}$$

= $m_{t} + 0.338 + 2.78\tau$ gms

as for the dampers used for the beam with distributed tuned dampers (case A). Again, graphs of $\omega_D^{\ m^1/2}$ against τ/h_D were plotted on the basis of measured values of ω_D for various m_t . This data is given in Table VIII. It is seen that the points on the graph of $\omega_D^{\ m^1/2}$ against τ/h_D in Figure 16 lie along the same line as for the dampers in case A, as would be expected. The small additional amount of viscoelastic material near the root of the cantilever damper contributes greatly to the damping but not to the damped natural frequency.

From Table 4 of Reference [4], the values of the test beam length L are obtained for the point of optimal tuning. A graph of ω_1 against L was obtained experimentally for the beam with a mass of 22 gms at the free end, and plotted in Figure 23. This mass represented the metal stamp used to ensure proper attachment of the cantilever damper at the free end of the test beam. Values of $(\omega_D/\omega_1)\sqrt{1+\psi}$ $(1+\eta^2)^{1/4}$ were then obtained from the test data, as in Table IX, and plotted against ψ_e in Figure 18.

(iii) Clamped-clamped beam with tuned damper at center under shaker excitation

This investigation has previously been reported in reference [5]. In this experimental investigation the effective damping was determined from the observed resonance amplification factor by the relationship $\eta = 1.32(A^2-1)^{-1/2}$. This setup is illustrated in Figure 24°,25. The beam length was now fixed at 19.9 inches, with a fundamental frequency of 90 cps. Graphs of η_s against the mass ratio ψ for several

values of η are given in reference [5] and are re-drawn as graphs of η_s against ψ_e (= 2.5\$\psi\$) in Figures 20 and 21, for η = 0.22 and 0.8 respectively. It is seen that the collapsed data is in good agreement with the theory and with the data for the cantilever beam.

The damper used in this investigation is illustrated in Figure 15C. Measurements of the loss factors and natural frequencies of the dampers were again made. The loss factor measurements are plotted in Figure 22. The value of \mathbf{m}_{D} is now:

$$m_D = 0.101 \times 1 \times 0.063 \times 454 \quad l_D$$
+ 0.0522 x 1 x $l_D \times \tau \times 454$
= (2.83 + 23.2 τ) l_D gms

where τ is the thickness of the viscoelastic material in inches, ℓ_D is the damper length in inches, the damper breadth is 1 inch. Graphs of $\omega_D^{m1/2}$ $\ell_D^{3/2}$ were again plotted against τ/h , on the basis of measured values of ω_D , in Figure 26. This data is given in Table X. The data for ℓ_D = 2.2 inches, 2.7 inches and 3.7 inches all fall on the same curve.

From Table 2 of Reference [5], the values of the damper mass m, needed for optimal tuning are taken and values of $(\omega_D/\omega_1)^{\frac{1}{2}} \sqrt{1+\psi}$ $(1+\eta^2)^{\frac{1}{2}}$ calculated as in Table XI. The data is plotted in Figure 18.

VI. CONCLUSIONS

A close approximation to the response of a single span beam with any boundary conditions, having isolated tuned viscoelastic dampers at arbitrary locations, under the action of any harmonically varying loading has been derived. Effective mass and stiffness parameters, and a system loss factor are defined. Comparisons are made with an exact theory of the response and damping of a clamped cantilever beam with a single tuned damper and an isolated harmonic force at the free end and a clamped-clamped, beam under shaker excitation, with a tuned damper at the center.

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TABLE 1

STANDARD INTEGRALS FOR VARIOUS BEAM CONFIGURATIONS

Boundary Conditions	Clamped - Free	Pinned- Pinned	Clamped- Pinned	Clamped- Clamped	Free-
ξ ₁	1.875	3.142	3.927	4.730	4.730
ξ ₁ 4	12.36	97.4	237.7	500.6	500.6
Δį	1.00	0.50	0.50	0.50	0.50
φ (Δ,)	1.000	1.000	0.957	1.000	1.000
$\begin{bmatrix} 1 & 1 & 1 \\ 0 & (\Delta) & \Delta \end{bmatrix}$	0.392	0.637	0.570	0.523	0.000
$\int_{0}^{1} \phi_{1}^{2}(\Delta) d\Delta$	0.250	0.500	0.439	0.397	0.250
$\phi_1^2(\Delta_1)/\int_0^1\phi_1^2(\Delta)d\Delta$	4.000	2.000	2.086	2.519	1.479
$\int_{0}^{1} \phi_{1}(\Delta) d\Delta / \int_{0}^{1} \phi_{1}^{2}(\Delta) d\Delta$	1.568	1.274	1.298	1.317	0.000

TABLE II

THEORETICAL RESONANT AMPLIFICATION FACTORS AND RESONANT

FREQUENCIES FOR CLAMPED-CLAMPED BEAM $(\alpha-1 = 0.317)$

Ψe	n	Γ _e = λ		orce Exc ak l	itation Peal		Shaker Ex Amplific	
		π 4	Ω	(ξ/ξ ₁) ⁴	Q (ξ/ξ ₁) ⁴	Peak 1	Peak 2
.1	•2	.031 .052 .083 .124	5.1 14.9 26.1	- .66 .80 .83	76.0 27.9 7.5 2.35	1.05 1.09 1.24 1.61	- 6.15 18.7 33.0	101 37.6 10.5 3.56
.1	• 5	.031 .052 .083 .124 .154	5.2 11.7 18.2	- .825 .85 .87	32.0	1.05	- 6.5 14.6 23.2	42.4 16.8 - -
.1	1	.031 .052 .083 .124 .154	10.0 16.6 22.6	- - .91 .89 .89	18.6 9.7 - -	1.03	12.7 21.0 29.1	24.6 12.8 - -
.1	1.5	.031 .0103 .0206 .0412 .062	14.8 53.2 23.5 11.8 11.9	1.01 1.01 1.01 .99 .93	= = = = = = = = = = = = = = = = = = = =	:	19.5 70.2 31.0 15.6 15.4	- - - -
.1	2	.0102 .031 .072 .154	41.0 13.8 17.3 39.2	1.01 .99 .91	=	- - -	54.2 18.1 22.2 51.0	= =
.2	• 2	.072 .103 .134 .165 .206 .268	3.9 5.9 9.5 16.2	- .52 .60 .66	31.5 15.6 8.8 5.4 3.2 1.8	1.11 1.18 1.28 1.40 1.61	4.5 7.0 11.5 19.9	42.5 21.4 12.4 7.82 4.85 2.91

TABLE II (CONT'D)

Ψe	η	$\Gamma_{e} = \frac{\lambda}{1}$	Fo: Peal	rce Exc	itatio Pea			xcitation cation A	teres de Miller de La
		π 4	Q	(ξ/ξ ₁) ⁴	Q (ξ/ξ ₁) ⁴	Peak 1	Peak 2	
.2	•5	.072 .103 .134 .165 .268 .206 .310	3.2 4.35 10.3 6.4 13.2	- .60 .66 .74 .70			3.82 5.25 12.7 7.82 16.40	17.80 9.50 5.60 3.70	
.2	1	.072 .103 .134 .165	4.86 4.99 6.30 14.7	- .99 .80 .78	7.60 - - - -	1.05	- 6.40 6.25 7.85 18.40	10.1	
. 2	1.5	.0414 .072 .103 .136	- 6.50 6.05 7.30	- .97 .87	11.8	1.03	8.52 7.72 9.25	15.7 - - -	
. 2	2	.0414 .072 .103 .134 .310	9.70 6.80 7.72 9.70 23.9	1.01 .91 .85 .83	- - - -	- - - -	12.8 8.75 9.82 12.20 30.40	- - - -	
. 4	.2	.103 .155 .206 .258 .310	1.75 2.46 3.41 4.76 6.45 9.70	.21 .31 .39 .43 .49	4.40	1.22	1.86 2.70 3.80 5.40 7.50 11.2	39.5 20.8 13.2 9.00 6.70 4.60	
. 4	. 5	.103 .155 .206 .258 .310	1.50 1.90 2.47 3.20 4.06	.23 .33 .41 .48	3.87 2.62	1.13 1.22 1.31 1.42 1.56	1.60 2.07 2.80 3.70 4.75	16.0 8.75 5.48 3.81 2.85	
. 4	1.0	.103 .155 .206 .258 .310	2.92 3.74 4.65	- .58 .60		1.09 1.12 - -	- 3.47 4.47 5.55	8.40 4.76 - -	

TABLE II (CONT'D)

Ψ́e	η =	Γ _e	Force Ex Peak l	citation Peak 2	Shaker Ex Amplific		
		π 4	Q (\(\xi/\xi_1\)	4 Q $(\xi/\xi_{1})^{4}$	Peak 1	Peak 2	
. 4	1.5	.072 .103 .155 .206	4.55 1.03 3.26 .79 4.02 .66	6.95 1.05 	6.05 4.10 4.88	9.30 - -	
. 4	2	.072 .103 .155 .206	5.56 1.01 4.00 .93 4.08 .72 5.20 .68 7.85 .68	 	7.20 5.10 4.95 6.30 9.55	- - - -	
.8	.2	.206 .290 .310 .352 .413 .454	3.14 .24 3.59 .269 4.03 .289 4.85 .330	9 7.20 1.51 0 5.65 1.61 0 4.90 1.68	2.38 3.37 3.90 4.33 5.36 6.10 7.90	23.0 14.5 12.4 10.6 8.60 7.52 6.07	
.8	• 5	.206 .290 .310 .352 .413 .454	1.70 .186 2.14 .269 2.27 .288 2.54 .309 3.00 .336 3.33 .353 4.05 .393	9 3.89 1.38 8 3.50 1.42 9 2.92 1.49 0 2.31 1.61 1 2.01 1.67	1.80 2.30 2.47 2.78 3.32 3.70 4.55	8.70 5.60 5.10 4.30 3.50 3.07 2.52	
.8	1	.206 .290 .310 .352 .413	1.74 .289 2.20 .350 2.32 .350 2.58 .393 3.03 .412	0 2.05 1.30 0 1.86 1.33 1 1.57 1.38	1.90 2.45 2.59 2.90 3.45	4.47 2.90 2.65 2.25 1.81	
. 8	1.5	.103 .165 .206 .290 .352 .413	2.44 .99 2.05 .392 2.65 .435 3.16 .455 3.72 .475	5	3.20 2.32 3.03 3.62 4.30	7.02 - 3.10 - -	

TABLE II (CONT'D)

Ψ́e	η	Γ _e	For Peak	ce Exci l		on ak 2		Excitation ication A
		$=\frac{\pi^{4}}{\pi^{4}}$	Q	(ξ/ξ ₁) ⁴	Q	(ξ/ξ ₁) ⁴	Peak 1	Peak 2
.8	2	.103 .165 .206 .290 .310 .352	2.44 2.47 3.25 3.46 3.90 4.55	- •99 •475 •475 •495 •497	4.0	2 1.05	3.20 2.84 3.75 4.02 4.52 5.28	5.35 - - - - -

TABLE III

THEORETICAL VALUES OF PARAMETERS FOR OPTIMAL TUNING
OF DAMPERS ON CANTILEVER BEAM UNDER FORCE EXCITATION

Loss Factor	VL	proxima	te Theo	ry		Exact	Theory	T.
n	$^{\psi}$ e	ηs	r _e ,	$\sqrt{\frac{e}{\psi_e}}$	Ψe	ⁿ s	Гe	$\sqrt{\frac{e}{\psi_e}}$
0.2	0.10	0.170	0.088	.94	0.08	0.165	0.073	.96
0.5		0.204	0.083	.91		0.150	0.065	.90
1.0		0.135	0.062	.79		0.110	0.055	.83
2.0		0.076	0.040	.63		0.070	0.032	.63
0.2	0.20	0.180	0.161	.90	0.20	0.185	0.161	.90
0.5		0.270	0.150	.87		-	-	-
1.0		0.250	0.112	.75		-	-	-
2.0		0.150	0.068	.58		-	-	-
0.2	0.40	0.190	0.290	.85	0.40	0.190	0.272	.83
0.5		0.380	0.252	.80		0.375	0.240	.78
1.0		0.420	0.200	.71		0.410	0.191	.69
2.0		0.285	0.124	.56		0.290	0.136	. 58
0.2	0.60	0.190	0.372	.79	1.60	0.190	0.580	.60
0.5		0.390	0.315	.73		0.435	0.520	.57
1.0		0.480	0.237	.63		0.610	0.365	.68
2.0		0.410	0.150	.50		0.680	0.227	.38
0.2	0.80	0.210	0.445	.75	0.80	0.190	0.425	.73
0.5		0.400	0.382	.69		0.405	0.366	.68
1.0		0.510	0.282	.60		-	-	-
2.0		0.505	0.176	.47		0.470	0.178	.47

TABLE IV

THEORETICAL VALUES OF PARAMETERS FOR OPTIMAL TUNING OF

DAMPERS ON CLAMPED-CLAMPED BEAM UNDER SHAKER EXCITATION

Loss		A	pproxim	ate The	ory		Exact	Theory
Factor	de	~	Г	1 e				Ге
η	Ψe	η _s	r e	VΨe	Ψe	ης	Γ e	VΨe
0.2	0.10	0.150	0.090	. 950	0.10	0.165	0.091	. 955
0.5		0.210	0.080	. 895		0.202	0.080	. 895
1.0		0.135				0.125	0.065	. 868
1.5		0.095				0.095	0.050	. 707
2. 0		0.080	⇔ ⊷	⇔ ⇔		0.083	0.038	.616
0.2	0.20	0.185	0.167	. 917	0.25	0.180	0. 205	. 910
0.5		0.310	0.148	. 862		0.321	0.170	. 830
1.0		0.280	0.116	. 760		0.293	0.140	. 750
1.5		0.200	0.088	. 663		0.230	0.115	. 678
2. 0		0.160	0.070	. 590		0.195	0.085	. 582
0.2	0.40	0.190	0.290	. 856	0.50	0.187	0.360	. 850
0.5		0.380	0.250	. 790		0.374	0.300	. 775
1.0		0.430	0.190	.670		0.460	0.240	. 692
1.5		0.400	0.160	.632		0.416	0.190	. 616
2. 0		0.350	0.111	. 528		0.378	0.150	.550
0.2	0.80	0.200	0.490	. 781	1.00	0.188	0.610	. 782
0.5		0.420	0.410	.717		0.403	0.480	. 692
1.0				-		0.580	0.350	. 590
1.5				-		0.600	0.270	. 520
2.0		0.550	0.200	. 500		0.600	0.220	. 469

TABLE V

LOSS FACTOR AND NATURAL FREQUENCY MEASUREMENTS FOR

TYPE A CANTILEVER DAMPERS

 $^{\ell}_{\rm D}$ = 1.125 ins., b = 0.50 in., h $_{\rm D}$ = 0.020 ins.

The state of the s	Statement of the second								
τ ins	input g's	m gm	Δ m = .338 + 2.08τ (gm)	m = m _t + Δm (gm)	ω _D Cps	ω _D π Cps (gm) 1/2	output g's	A	n = 1/\A ² - 1
0.02	000000000000000000000000000000000000000	221211100011100011000110000110000110000110000	0.4	01111122 01114872004 0444400040404	74448888874 7999888878 7999888878	180 152 152 169 164 181 181	22 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	01111111 01120111 0112001100777 0121200011	102 000 062 0050 062 082 106 133
0.035	NNNNN00000	9.5 111.0 118.0 23.0 27.0 27.0 5	0.4	1119.9 118.4 118.9 118.9 118.9 123.9	2004433193374 2004433193375	171 163 169 150 167 147 132	3.5 3.5 3.5 3.5 1.7 1.0 1.0 1.0 1.0 5.0	116 7 7 8 8 8 8 8 9 8 8 9 8 9 8 9 8 9 8 9 8	158 129 126 126 119 126

TABLE V (CONT'D)

tins	input g's	m gm	Δ m = .338 + 2.08τ (gm)	m = m _t + Δm	e D C D S	w D m Chs (gm) 1/2	outrut g's	A	n = 1/√ <u>A² - 1</u>
. 062		84444444444444444444444444444444444444	ن • 0	01111122 111120 011142000000000000000000000000000	0 4 4 0 4 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	11111111111111111111111111111111111111	800 ••• m N m	0.00	.132 .175 .169
0.125	0000000000	22233 2223 2223 2223 2223 223 223 223 2	9 0	22212222222222222222222222222222222222	84 48 60 60 60 60 60 60 60 60 60 60 60 60 60	283 233 233 223 224 217	33.7 33.2 33.2 20.0 20.0 30.0 80.0 80.0 80.0 80.0	4 0 0 4 4 4 0 4 4 4 4 6 6 6 6 6 6 6 6 6	233 253 253 253 253 233 233

TABLE VI

OPTICAL DAMPING MEASUREMENTS FOR TYPE A TUNED DAMPERS

TABLE VII

VALUES OF $(\omega_{\rm D}/\omega_{\rm l})$ $(1+\psi_{\rm e})^{1/2}$ $(1+\eta^2)^{1/4}$ for optimal funing of cantilever beam

WITH 11 DISTRIBUTED TUNED DAMPERS (TYPE A) UNDER SHAKER EXCITATION

TEST BEAM THICKNESS h = 0.25 inches

 $\eta = 0.175$, $(1+\eta^2)^{1/4} \approx 1$, Temp $\approx 80^{\circ}F$ $\psi = \psi_e$, $\ell_D = 1.125$ in., b = 0.5 in., $h_D = 0.02$ in.

" s = //AZ-1	050	.112 .139 .140	11.8 11.2 11.2 11.7	.100
A	15.2 19.8 15.6	9.07.7.3	യ യ യയയ സതഠരം	10.2
ligh Freq	7.60 9.40 7.80	4.40 7.30 3.60 8.80	8.50 4.40 4.50 8.60	5.10
T III	139	109 126 128 116	114 112 176 170 174	124
ow Freq peak eq output ps g's	7.60 9.40 7.80	4.50 7.20 3.60 8.70	8.50 4.30 4.50 8.60	5.10
Low fred cps	89	47 60 59 48	49 49 101 102 104	100
input g's	0000	0.0	0000	0.5
$\frac{\omega}{\omega_1}\sqrt{1+\psi}$.985 .910 1.01	0.96 1.02 1.02 0.96	1.00	1.07
3 3	.221 .650 .605	.640 .412 .412 .640	650 650 236 236 236	700
w _l	119 85 87	8008	84 131 133 133	94
L ins	7.30 8.92 8.78	9 00 00 00 00 00 00 00 00 00 00 00 00 00	88 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	8,40
°D cps (Fig 1	106	62 77 77 62	66 66 117 117	177
m = m _t +Δm gm	5.72	0 0 0 0	00000	8.1
ωδ gm	4	ro.	r.	9.
m gm [9]	2.1	7.5	22.7.2	7.5
ins	.020	* *	.078	.125

* Additional data not included in [9].

TABLE VIII

LOSS FACTOR AND NATURAL FREQUENCIES OF TYPE B CANTILEVER DAMPERS

 $\ell_{\rm D} = 1.125 \; {\rm ins.}, \; {\rm b} = 0.50 \; {\rm ins.}, \; {\rm h}_{\rm D} = 0.020 \; {\rm ins.}$

				-	Total Control of the					
τ ins	input g's	m _t gm	Δm = 0.333+2.78τ gm	m = m _t + Δm gm	e CDS	wb/m cns gm1/2	Temp °F	Output g's	A	$n = \frac{1}{\sqrt{\Lambda^2 - 1}}$
.062	0.40	37 37 37	0.51 0.51 0.51	37.5 37.5 37.5	28 29 26	171 178 159	77 82 82	1.32	3.30 3.71 3.34	.318 .280 .316
3.00.	00000000000000000000000000000000000000	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		0.000000000000000000000000000000000000	00000000000000000000000000000000000000	1120 1120 120 120 120 120 120 120 120 12	000007770000	11111111111111111111111111111111111111		4 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
. 125	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.000000000000000000000000000000000000	000000000000000000000000000000000000000		######################################	196 196 196 172 172 196	0000000 000000000000000000000000000000	1.65 2.32 2.32 1.33 2.21	447 647 277 8 677 77 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	243 216 176 316 2223

TABLE IX

VALUES OF (w_D/w_l) (1+v_e)^{1/2} (1+n²)^{1/4} FOR OPTIMAL TUNING OF CANTILEVER BEAM WITH

TYPE B TUNED DAMPER AT FREE END, UNDER SHAKER EXCITATION (FROM [4])

TEST BEAM THICKNESS = 0.125 ins.

 $\psi_{\rm e}$ = 4 $\psi_{\rm v}$ $\ell_{\rm D}$ = 1.125 ins., $h_{\rm D}$ = 0.02 ins., input = 0.5 g's

ins	⋺	# pu	m mg	m = m _t +Δm	wP,m (Fig	cps (Fig	Ins	e C C C C C C C C C C C C C C C C C C C	ω ₁ /1+ψ _e	Temp °F	Low freq cps	w Freq	High ped freq cps	r Frequak output g's	1.56 \A ² -1
0 (325,0=n)	257	15.75 18.0 20.3	512	16.2 18.5 20.7	176 176 176	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	50 80 80 80	72 64 64	8 9 9 8 5 8 8 5 8	77 78 77	33 31 31	2.49 2.49 2.40	82 76 73	2.70	.306 .310
· (04.0=n)	207306	2.25 4.50 6.75 9.00 11.25 20.25		785100	88711887	40000	014000	84408F	.71 .74 .79 .83 .86	81 82 82 83 83	112 81 63 53 45	2.36 2.27 2.05 2.30 2.48 1.90	195 152 130 114 102	2 4 3 2 4 4 3 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	334 342 372 356 403
0.125	0 6 1	• •		1.0	2 2 2		m m œ		0000	8 8 8 8 0 8			00 00 10		20 21 21 21
(22°0=	070	100	999	7.3	222		862	108	000	8 8 8 8 7					220
=u)	223 200 300 200 200 200 200	13.5 15.75 18.0 20.25	0000	14.1 16.4 18.6 20.9	0 0 0 0 0 0 0 0 0 0 0 0 0 0	5 5 5 4 8	55.08	81 76 70 68	1.03	8 8 8 8	3 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	9 9 8 8 9 6 2 9 0		. 226 . 232 . 244 . 251

TABLE X

LOSS FACTORS AND NATURAL FREQUENCIES OF TYPE C TUNED DAMPERS

 $h_{\rm D} = 0.063 \; {\rm ins.}$, $b = 1.0 \; {\rm ins.}$

م ر ا	r i	+ te	Δm	m = m. + \Dm	a D	w_m1/2 g_3/2	Temp	input	output		F
		,	'n	t gm	cps	g _ g	் பு o	g . 8	ຊູ ຣ	K	$1/\sqrt{\Lambda^2-1}$
3.7	0.020	7.6	2.9	10.5		63		0.5		000	50 1
					71	1635 1635	0 7 8		2 Q	10.01	053
						99			•	6	5
	0.035	7.6	3.2	10.8	71	1660	79	0.5	7.32	15.64	.064
					71	99	21		. 2	9	9
	0.062	7.6	3.7	11,3		89		0.5		0	6
					71	1700	79		5.2	10.4	960.
						74			•	٦.	00
	0.078	7.6	4.3	11.9	77	8		0.5			
					72	1740	81		3.90	7.3	.129
						2					3
						74			•		N
	0.125	7.6	5.0	12.6		90		0.5	.2	4.5	.226
						02			.2		2
					79	1990	80		2.3		22
						02			.7		2
						66			۳		21
						96			•	•	-
STORE LEGISLATION	PRINCIPLE AND AND AND ADDRESS OF THE PRINCIPLE AND ADDRESS OF THE PRINCIPL	THE NUMBER OF PERSONS ASSESSED.	The state of the s		AND RESTREET SPECIAL PROPERTY.						

TABLE X (CONT'D)

$n = 1/\sqrt{N^2 - 1}$.044	.114	.196 .203 .178	.046 .039 .044	.138
A L	22.4 32.3	8.8	5.0	21.8 25.7 23.0	7.44
output g's	11.2	4.4 7.4 3.5	2.2	10.01 7.77 11.55 9.9	3.72
input g's	0.5	0.5	0.50	0000	0.5
Temp	78	83 76 76	76 76 79	888 84 84	80 76
ω _D m1/2 ε _D 3/2	1670 1670	1380 1730 1780	2040 2040 1840	1750 1750 1670 1670	1780 1800
w _D cps	83	92 80 82	91 91 81	72 72 78 78	72
m = mt+∆m	38.5	39.6 44.4 44.4	48.8	30.1 30.1 23.3 23.3	31.1
Δm	1.7	2.6	3.0	2.1	3.1
m+ gm	36.8	37.0 41.8 41.8	45.8	28.0 28.0 21.2 21.2	28.0
ıins	0.020	0.078	0.125	0.020	0.078
% D ins	2.2			2.7	

TABLE XI

VALUES OF (ω_D/ω₁)(1+ψ_e)^{1/2}(1+η²)^{1/4} FOR CLAMPED-CLAMPED BEAM WITH TUNED DAMPER

AT CENTER UNDER SHAKER EXCITATION (FROM [J])

TEST BEAM THICKNESS = 0.217 inches $\psi_{\rm e} = 2.5\psi, \ \omega_{\rm l} = 90 \ \rm cps$ $Temp = 80°F \ (*3°F)$

req tred treut d's	7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	24 50 50 50 50 50 50 50 50 50 50 50 50 50
High Freq peak freq output cps q's	107 106 107 107 107 108	003 000 002 003 003 001 003
s at	15.00 15.00 15.00 15.00	23.3 2.0 2.0 2.0 2.0 3.0 5.0 1.0 6.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7.0 7
Low Freq peak freq outpi cps g'	73 72 71 70 67 68 67 65	30 77 76 75 76 75 74 73
input g's	0.3 0.5 2.0 1.0 0.75	00.00
$\frac{^{\omega_{D}}}{^{\omega_{1}}}\sqrt{1+\psi_{e}}$ (1+ $^{\eta_{2}}$) 1/ 4	1.06 1.03 1.08 1.08 1.09 1.09	1.20 1.11 1.07 1.03 1.05
ψ = m/396	042 052 049 075 062 068 073	.014
«D Cps	00000000000000000000000000000000000000	93 91 91
ω _D λ _D /2 /m Fig. 26	1660 1740 1800 2000	1660
m gm	16.6 19.8 19.5 29.8 27.5 24.0 26.9 31.9	2
Δm gm	33222	7 7 8 8 7 Y Y Y Y Y Y Y Y Y Y Y Y Y Y Y
m gm	14.5 117.7 116.5 23.0 23.0 25.5 25.5	2 2 2 8 8 4 9 6 4 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9
r ins	0.020 0.062 0.073 0.125	0.020 0.035 0.062 0.073 0.125
ins ² D	2 . 7	3.7

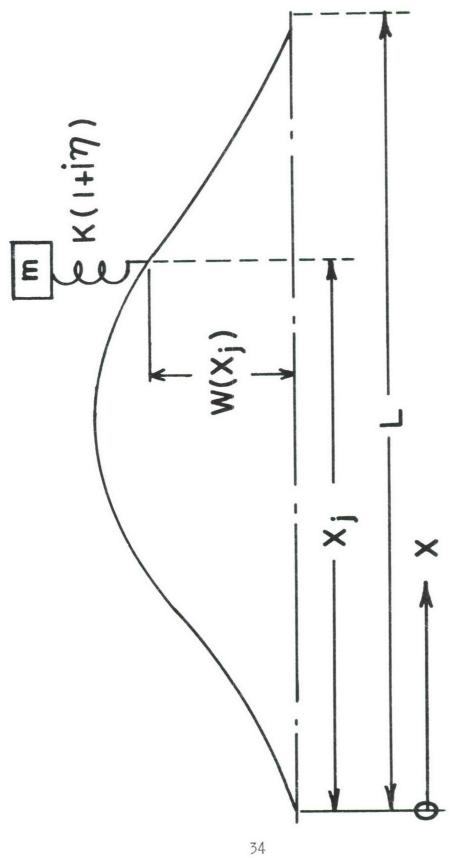


Figure 1. Idealized Beam - Damper System

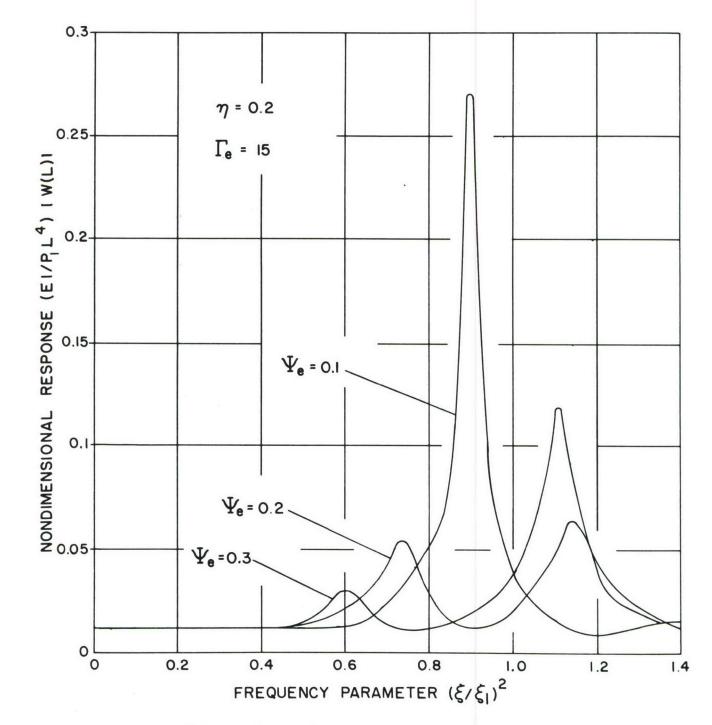


Figure 2. Typical Response Spectra for 7 = 0.2

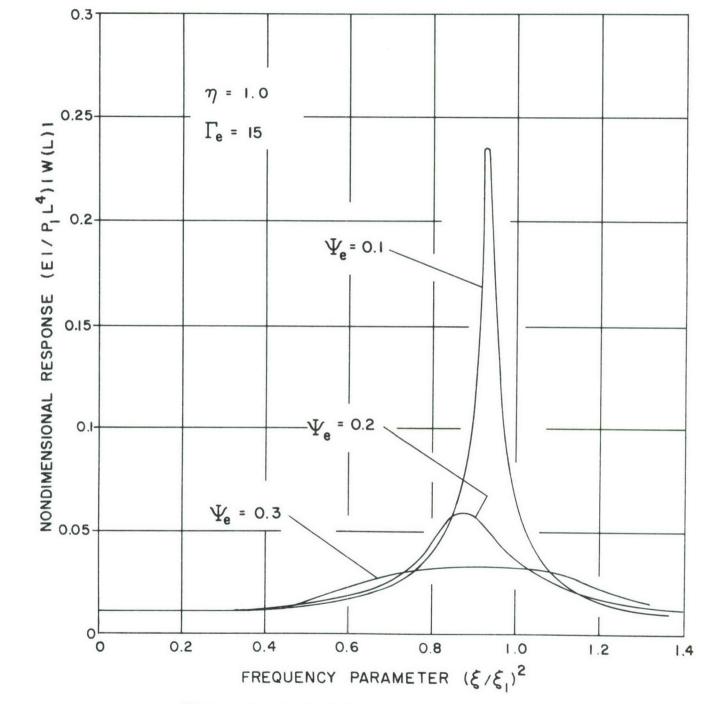
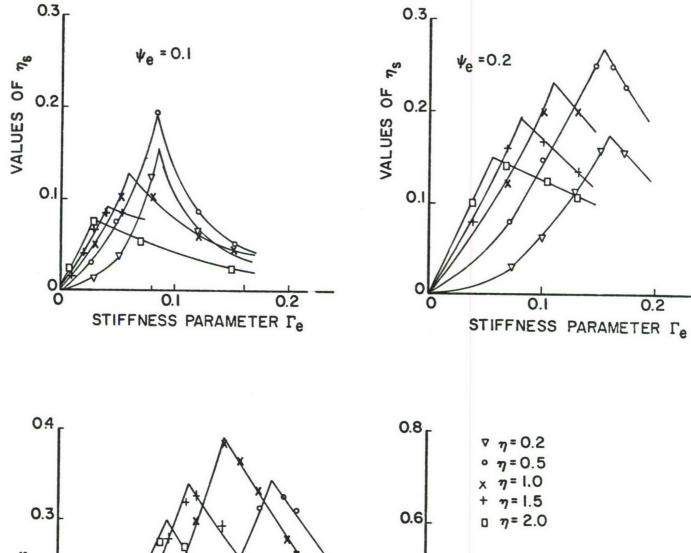


Figure 3. Typical Response Spectra for η = 1.0



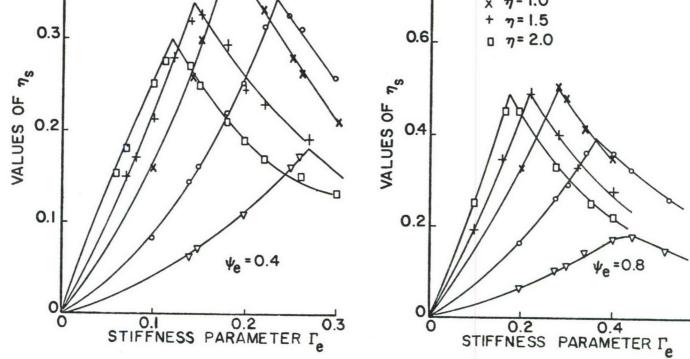


Figure 4. Graphs of Effective Loss Factor $n_{_{\rm S}}$ Against Stiffness Parameter $\Gamma_{_{\rm C}}$ (Force Excitation)

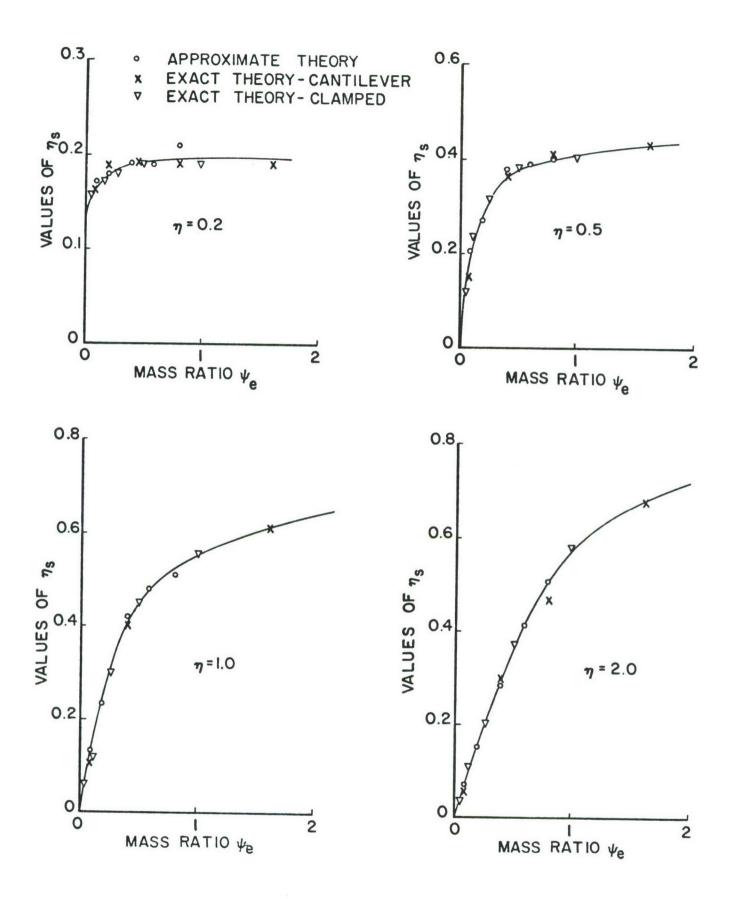


Figure 5. Graphs of Optimum Effective Loss Factor Against Effective Mass Ratio $\psi_{\mbox{\scriptsize e}}$

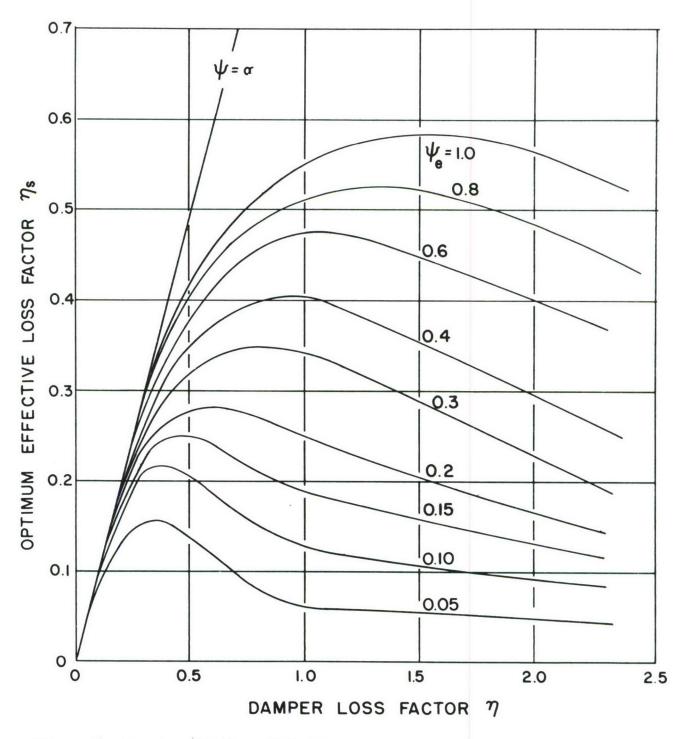
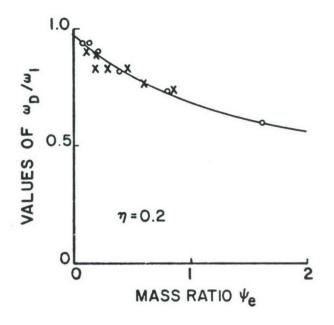
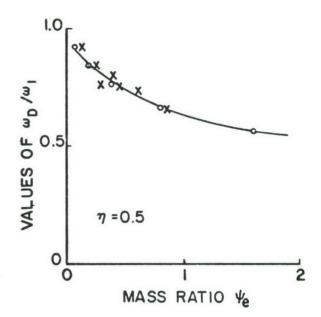
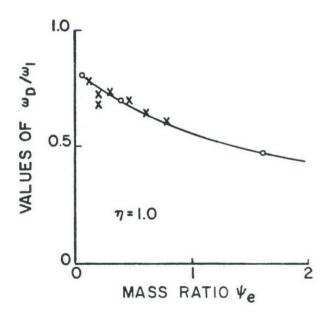


Figure 6. Graphs of Optimum Effective Loss Factor Against Damper Loss Factor





- · APPROX. THEORY
- x EXACT THEORY (CANTILEVER)



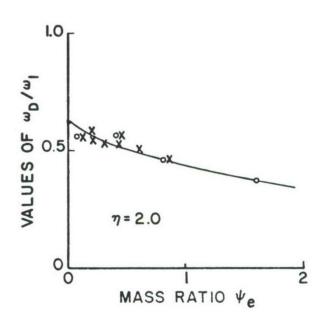
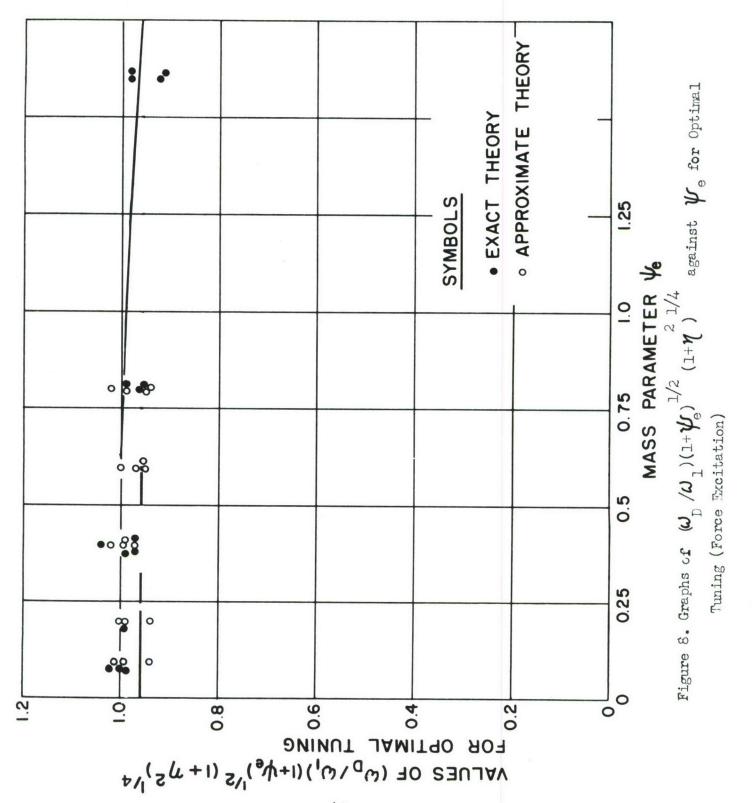


Figure 7. Graphs of ω_D/ω_1 Against ψ_e for Optimal Tuning (Force Excitation)



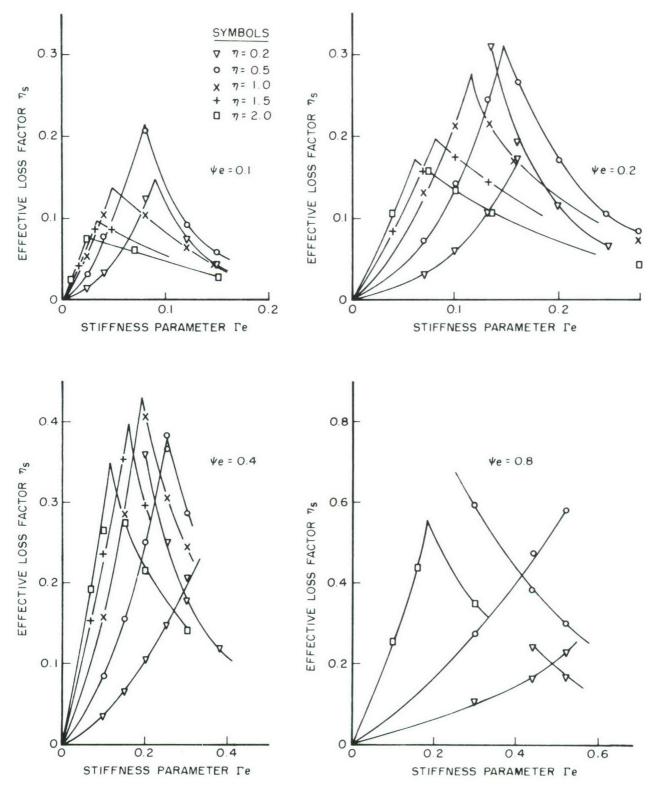
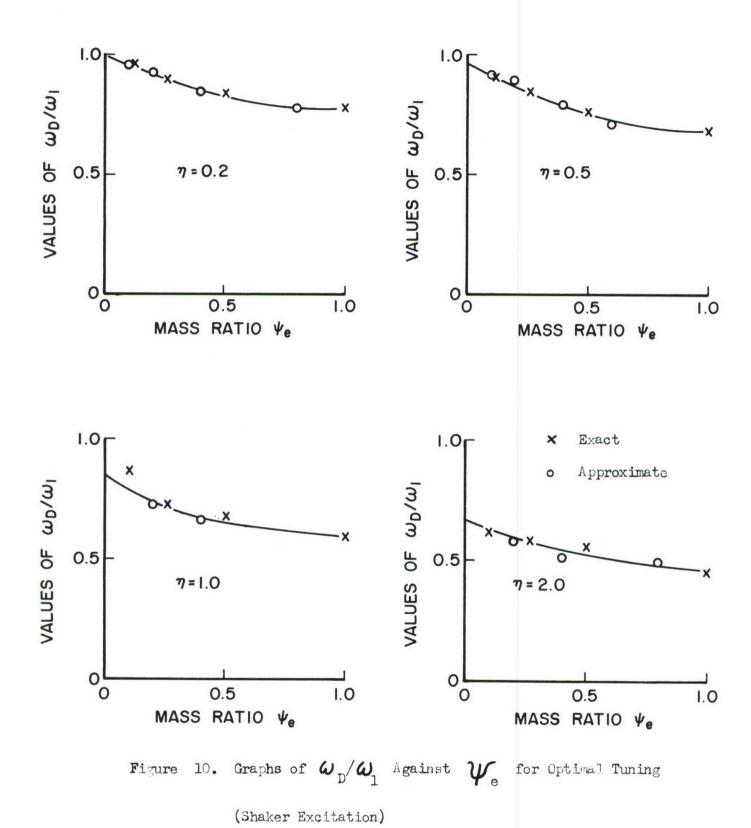


Figure 9. Graphs of Effective Loss Factor 7s Against Stiffness Parameter $\Gamma_{\rm e}$ (Shaker Excitation)



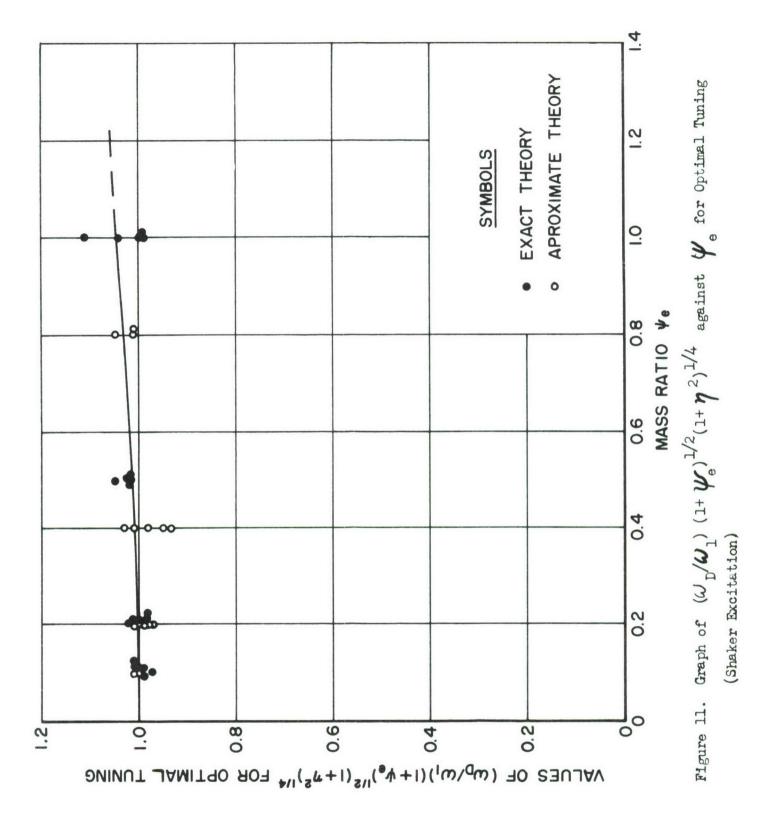
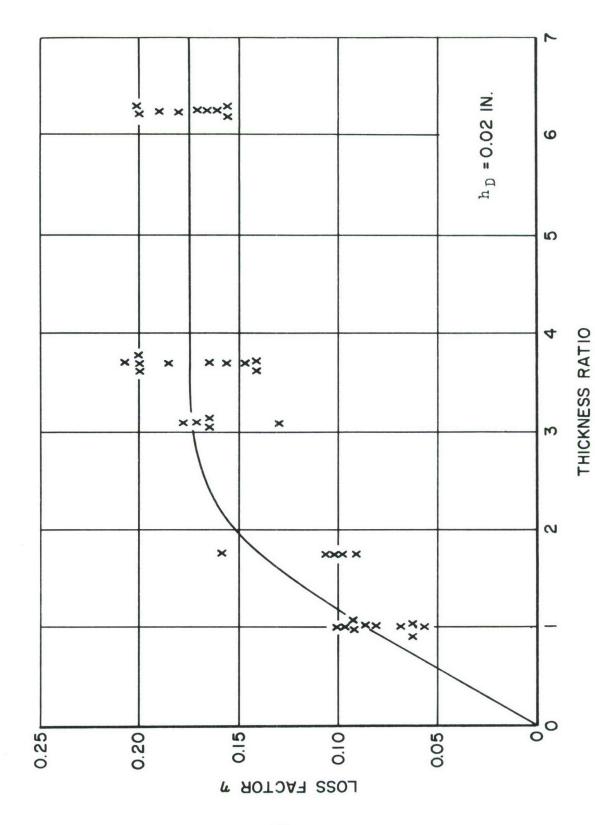


Figure 12. Photograph of Beam-Damper System



Graph of Damper Loss Factor η against Thickness Ratio $\sqrt{Lh_D}$ for Cantilever Dampers Used as Distribution on Cantilever Beams Figure 13.

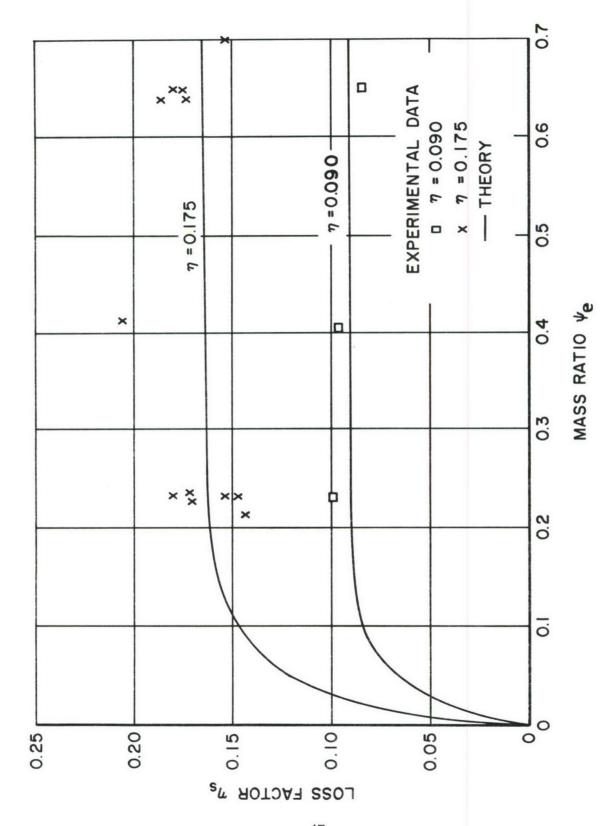
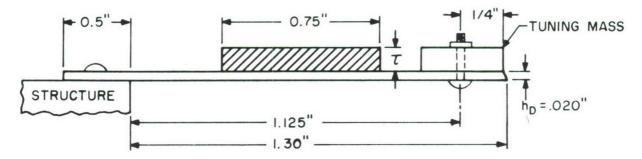
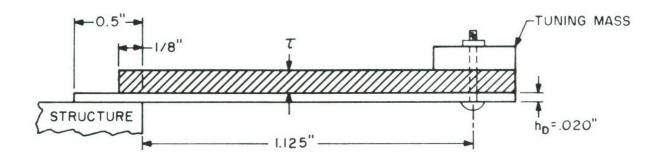


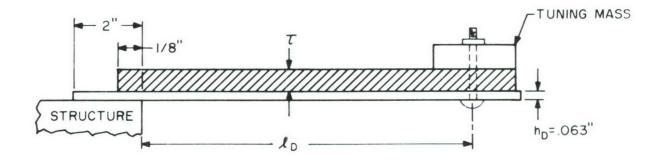
Figure 14. Experimental Values of $\eta_{\rm s}$ Plotted Against $\psi_{\rm e}$ for η = 0.175 and 0.09



(a) LOW LOSS FACTOR UNITS USED ON CANTILEVER BEAM (DISTRIBUTED DAMPERS)



(b) HIGH LOSS FACTOR UNITS USED ON CANTILEVER BEAM (SINGLE DAMPER AT FREE END)



(c) DAMPER UNITS USED ON CLAMPED-CLAMPED BEAM

Figure 15. Sketches of Tuned Dampers Used in Experimental Investigations

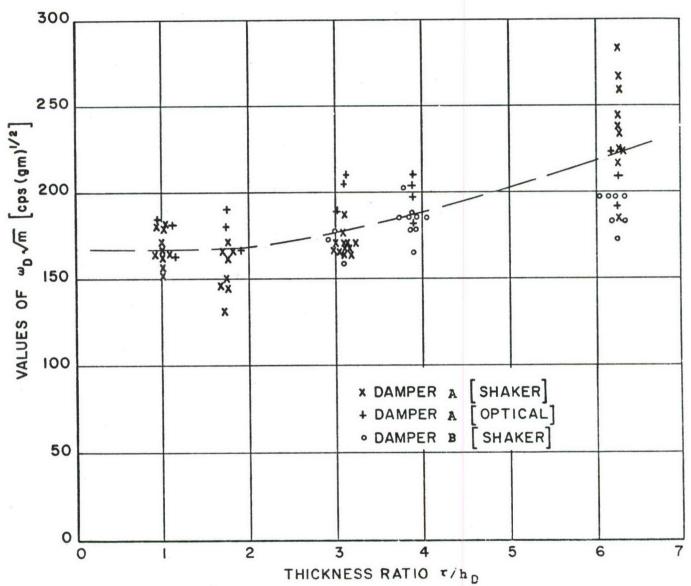


Fig 16.Graph of $\omega_D^{m1/2}$ Against τ/h_D for Tuned Dampers Used on Cantilever Beams (Both Distributed and Singly at Free End)

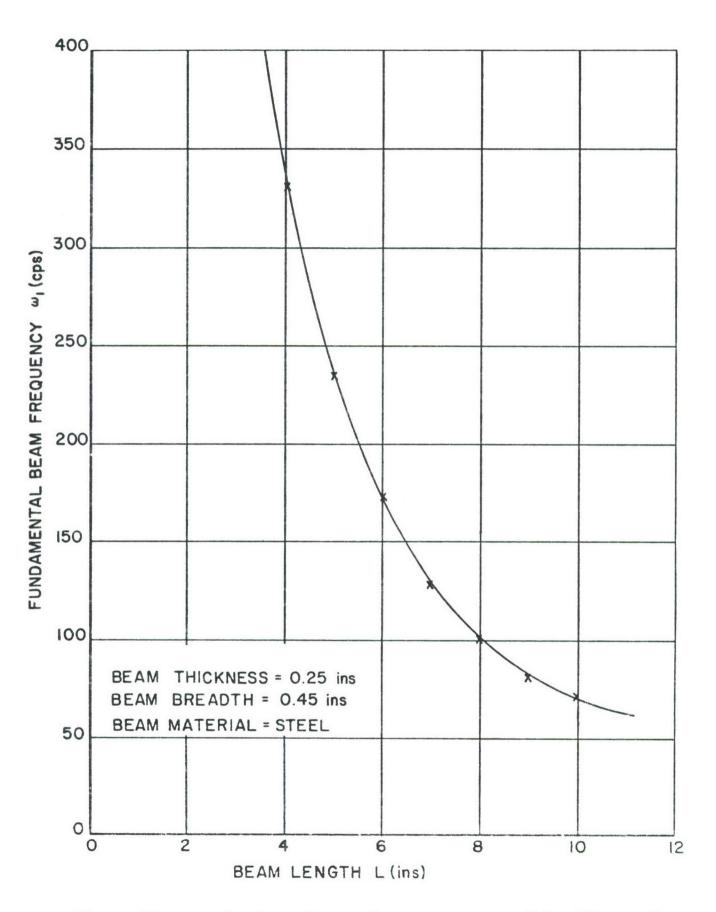
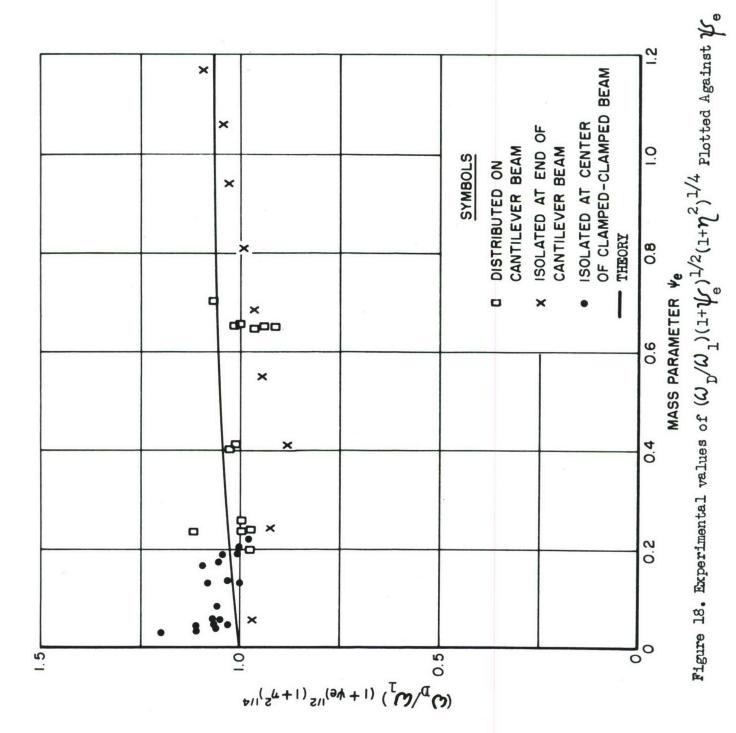
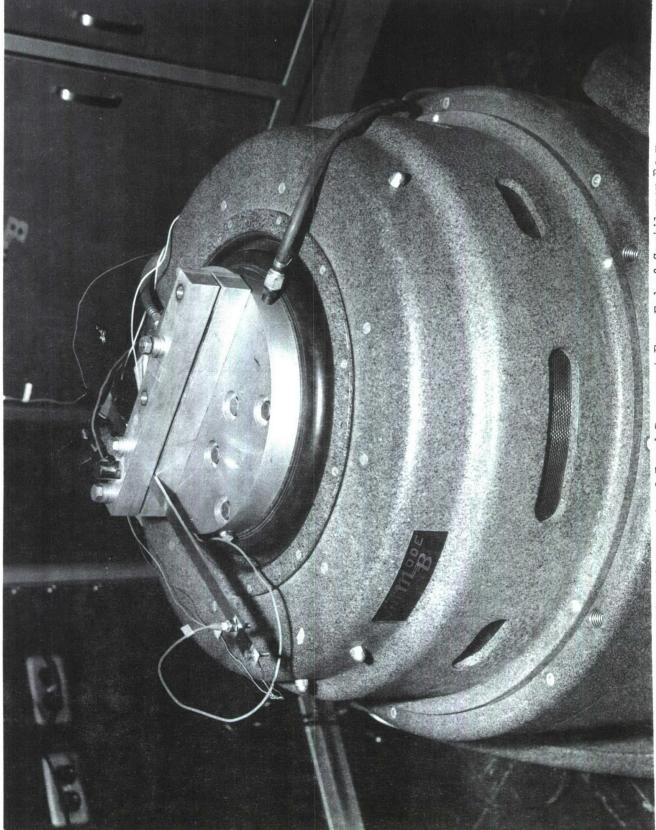


Figure 17. Craph of Fundamental Frequency ω_1 of Cantilever Beam Against Length L





Photograph of Tuned Damper at Free End of Cantilever Beam Figure 19.

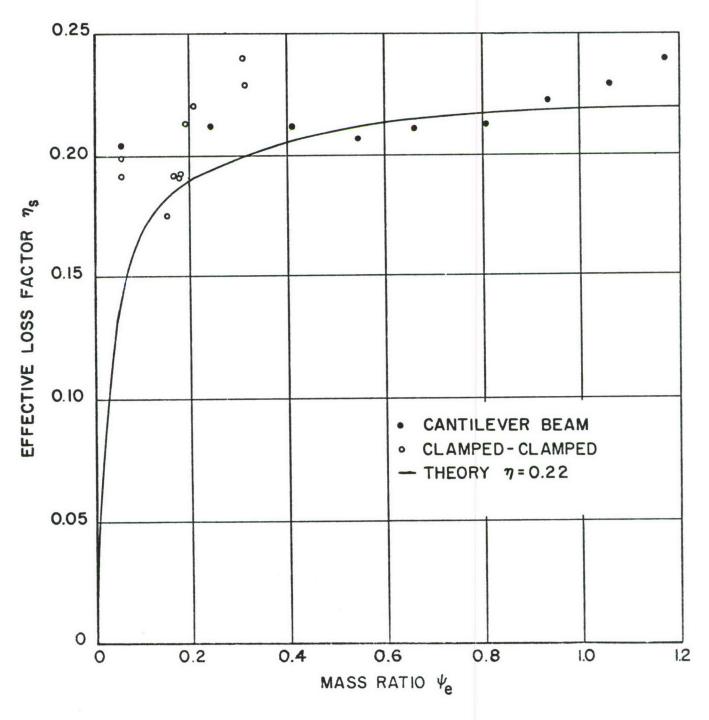


Figure 20. Experimental Values of $\ensuremath{\eta_{\mathrm{S}}}$ Plotted Against $\psi_{\ensuremath{\mathrm{e}}}$

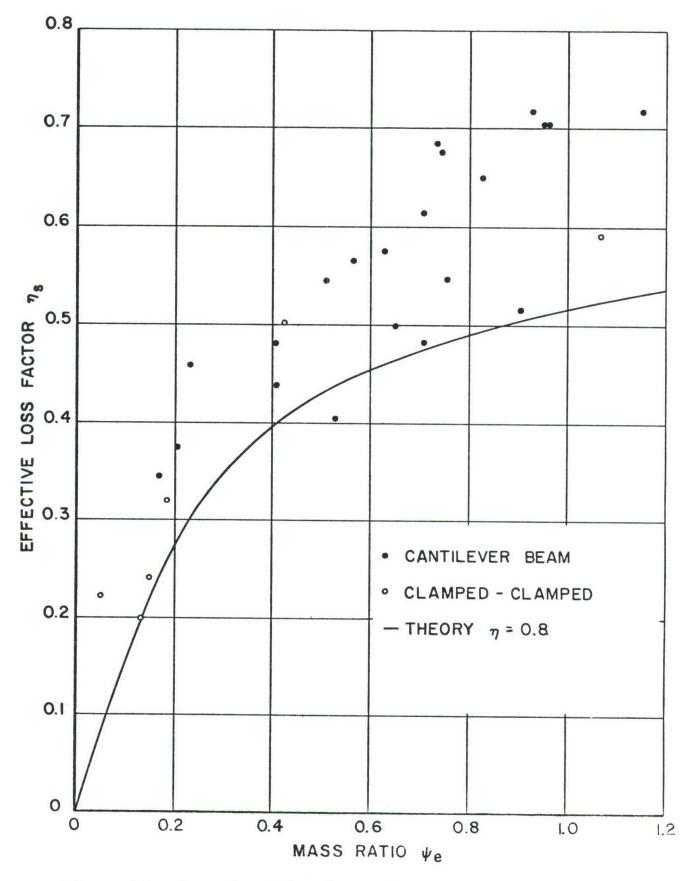
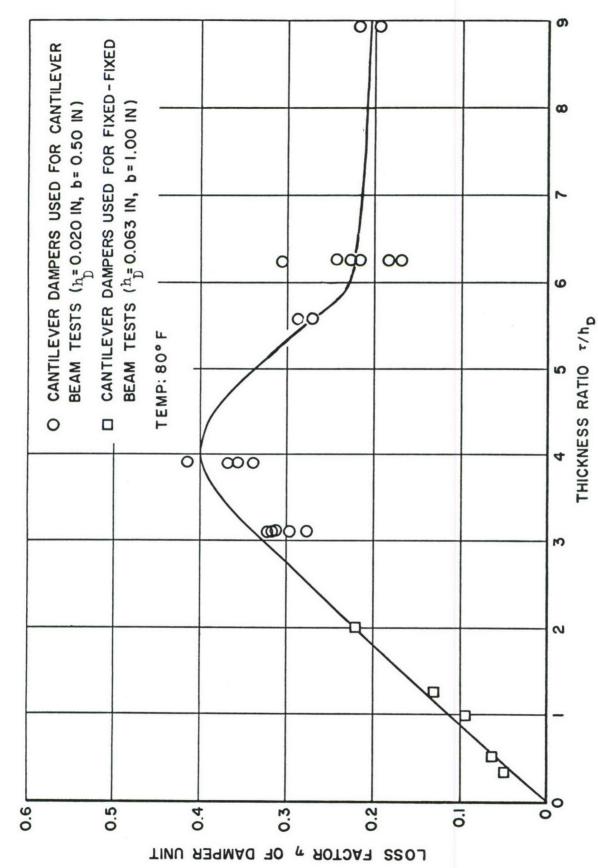


Figure 21. Experimental Values of n_s Plotted Against ψ_e



Graph of cantilever damper loss factor n against ratio of viscoelastic layer thickness to thickness of metal cantilever (80°F.) Figure 22.

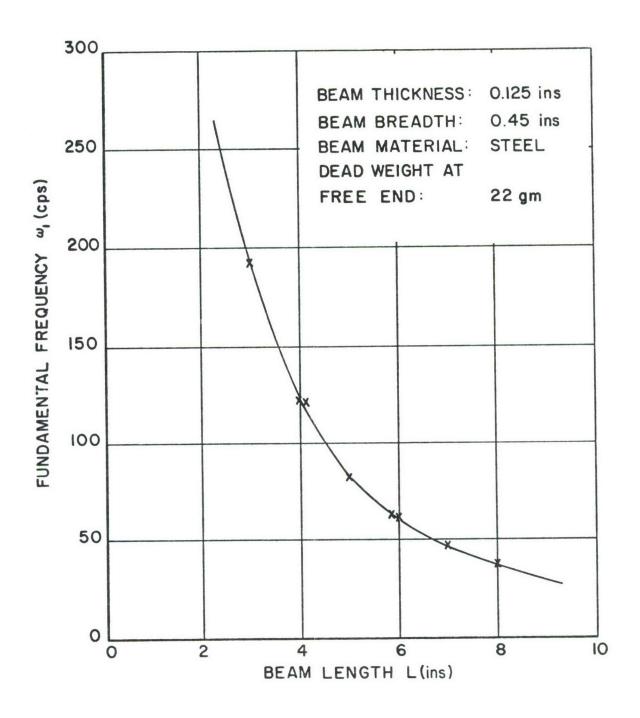


Figure 23. Graph of Fundamental Frequency ω_1 of Cantilever Beam Against Length L (Dead Weight of 22 gm at Free End)

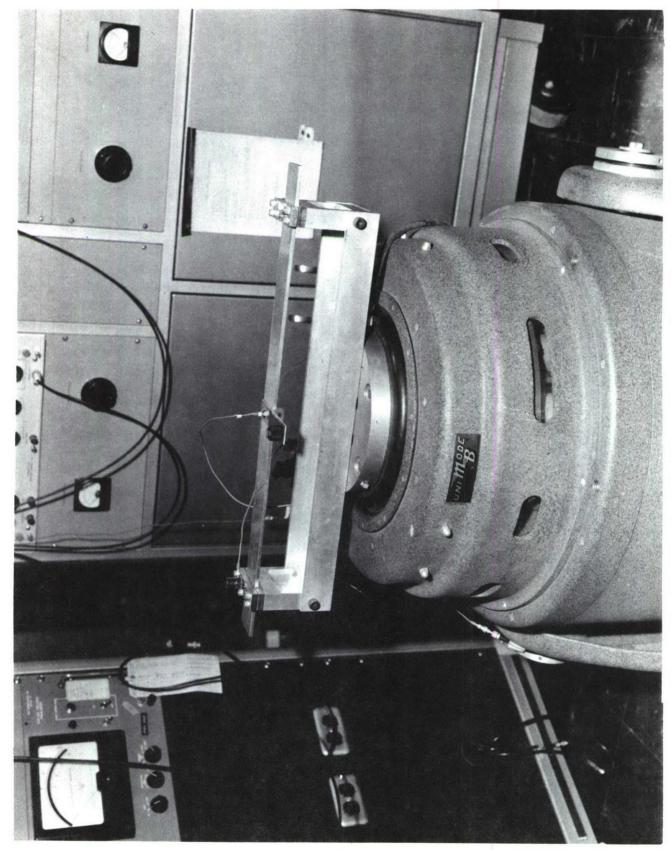


Figure 24. Photograph of Cantilever Damper on Clamped - Clamped Beam

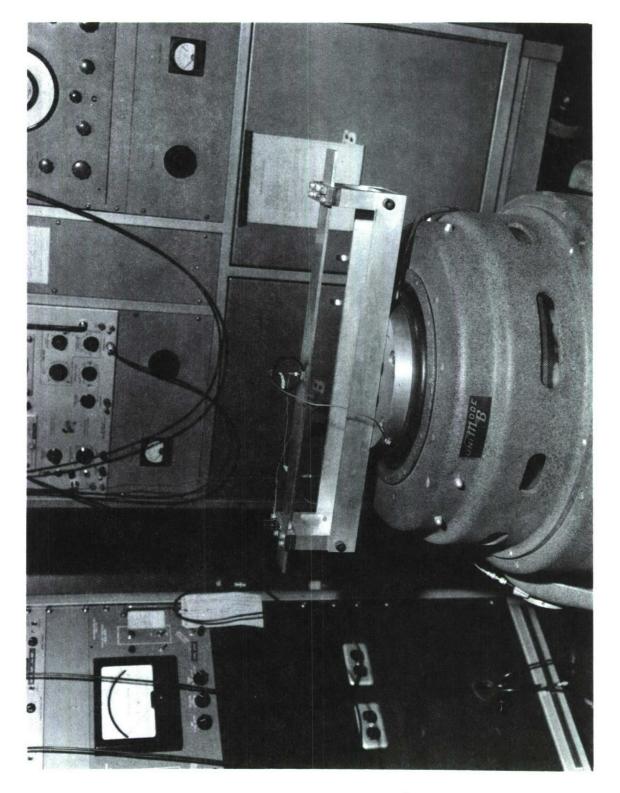


Figure 25. Photograph of Ring Damper on Clamped-Clamped Beam

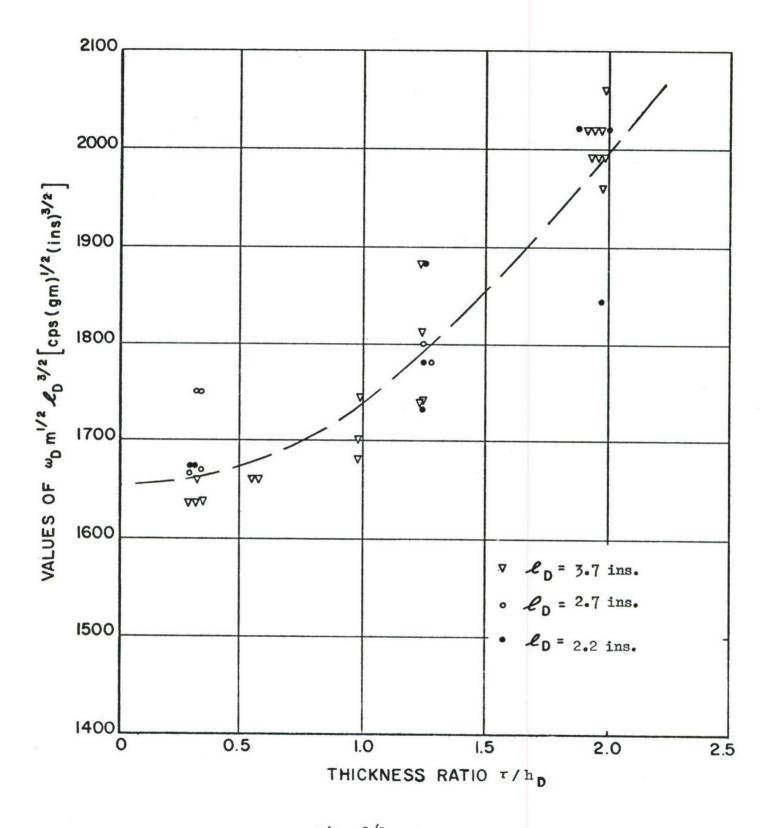


Figure 26. Graph of $\,\omega_{\rm D}^{-1/2}\,\ell_{\rm D}^{3/2}\,$ Against $\,\tau/{\rm h}_{\rm D}^{}$ for Tuned Dampers Used On Clamped-Clamped Beam

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13. ABSTRACT

An approximate analysis of the response of the fundamental mode of any simple single span beam with tuned viscoelastic dampers attached at discrete locations to a harmonic loading with arbitrary spatial distribution is derived. It is shown that, to a good degree of approximation, a single expression can be made to represent the response in the fundamental mode of a beam with any boundary conditions, provided that certain effective mass and stiffness parameters are defined for the beam-damper configuration. Comparisons are made with experiments and with an exact theory, subject to the limitations of the Euler-Bernoulli beam equation, of the response and damping of a cantilever beam having an isolated harmonically varying load at the free end and a clamped-clamped beam, with a tuned damper at the center, under shaker excitation. Good agreement between the exact and approximate theories and the experiments is demonstrated. Conclusions are drawn concerning the equivalent damping introduced into the simple structure by the tuned dampers and the damper natural frequency needed for optimal damping.

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